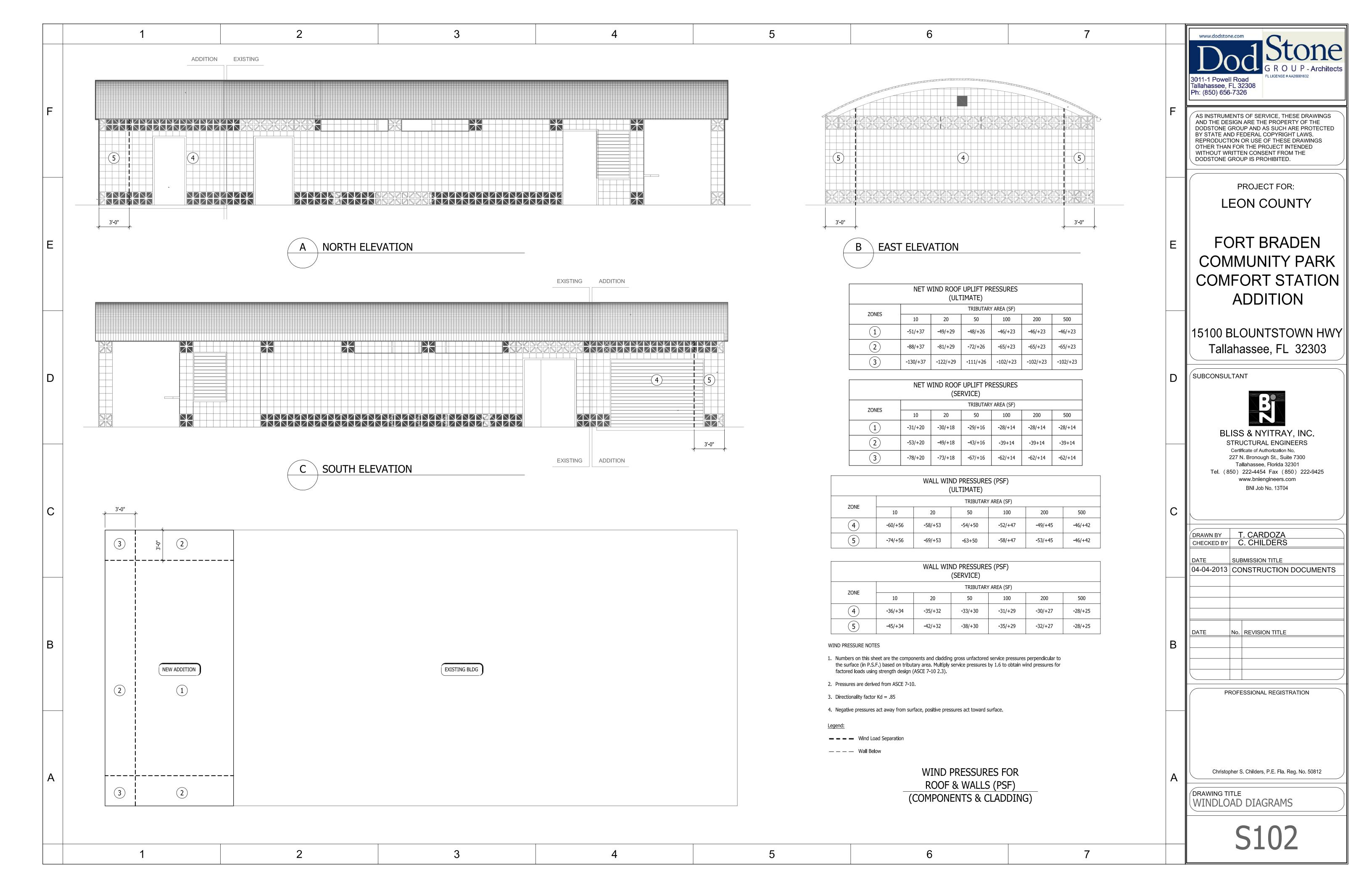
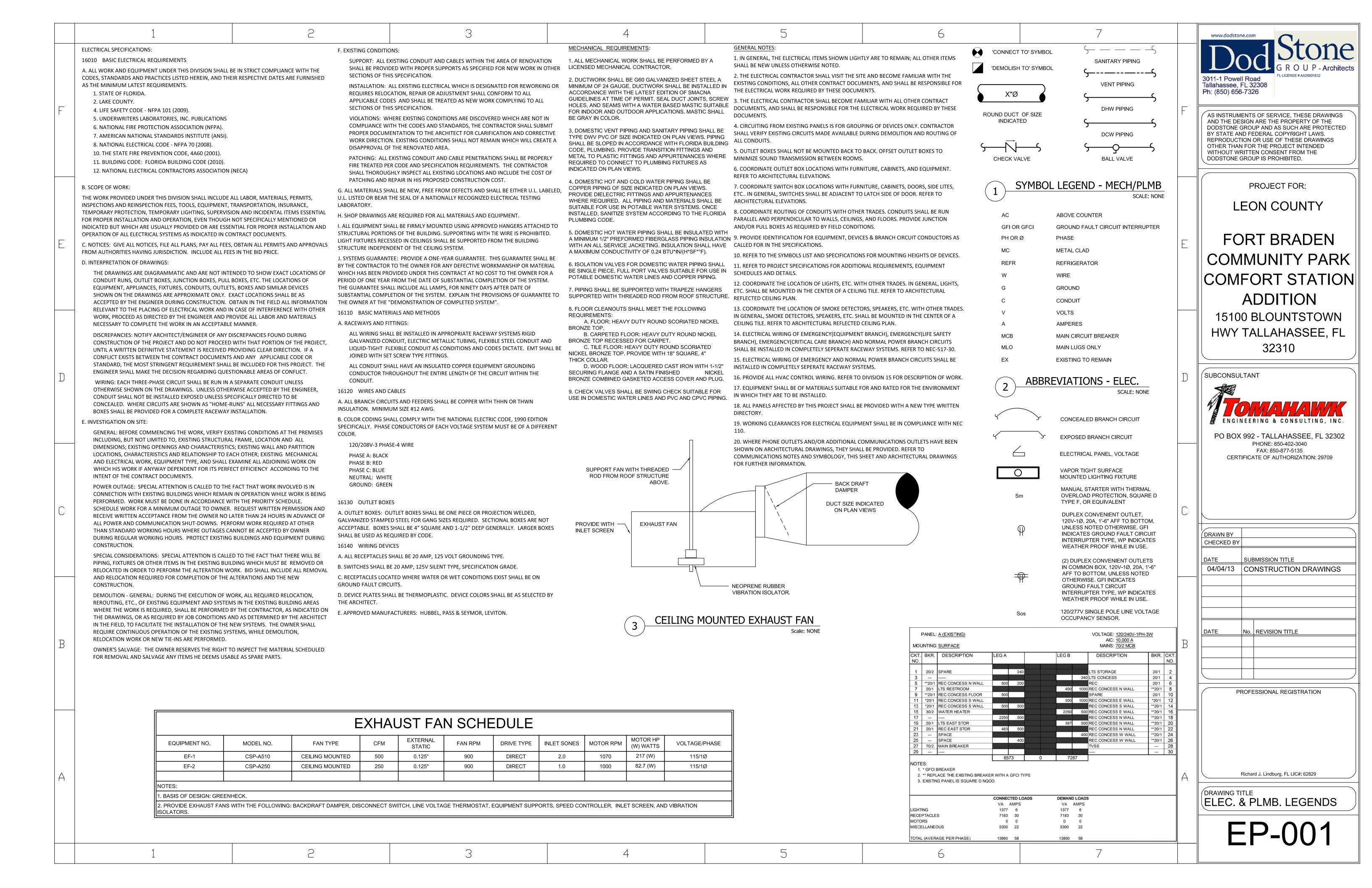
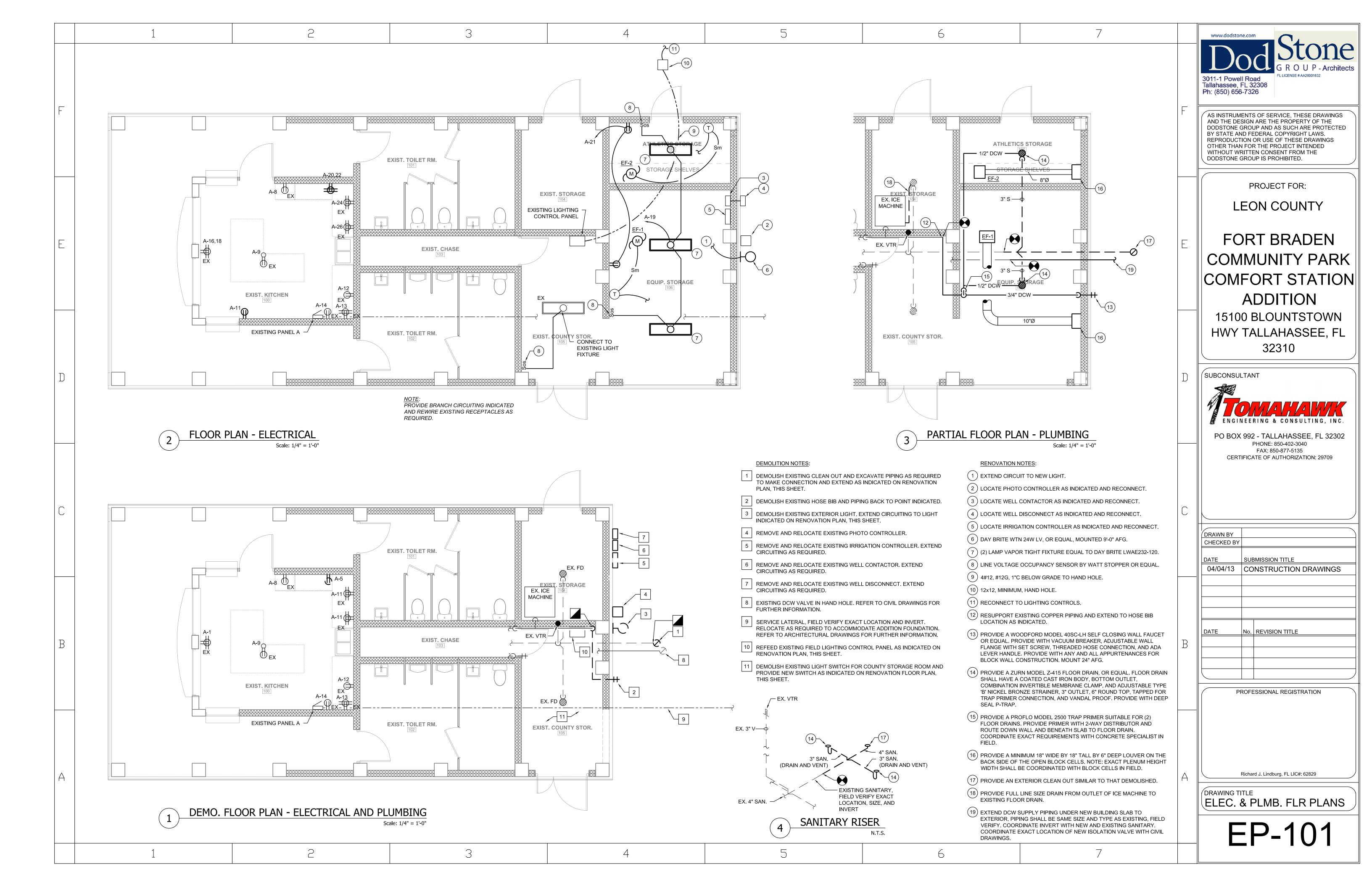


		T 0		www.dodstone.com
				DOC GROUP-Arch
		STRUCTURAL NOTES		3011-1 Powell Road Tallahassee, FL 32308 Ph: (850) 656-7326
	z a GENERAL NOTES		8. Where reinforcing steel congestion permits, conduit and pipes up to 1" diameter may be embedded in concrete per ACI 318,	
	 The governing Code for this Project is the Florida Building Code, 2010 Edition. This Code prescribes which edition of each referenced standard applies to this Project. 	 SHALLOW FOUNDATIONS Footing sizes and reinforcing are based on an assumed allowable soil bearing capacity of 2000 psf. All footings shall bear on soil or rock prepared per the Geotechnical Engineers Report. The Contractor will engage a Florida licensed Geotechnical Engineer to 	Section 6.3. Space at 3 diameters o.c. Place between outer layers of reinforcing if conduits are significantly congested, additional reinforcing perpendicular to piping may be required. Requests to embed larger pipes should be accompanied by a detailed description and be submitted to the architect for evaluation.	AS INSTRUMENTS OF SERVICE, THESE DRAWIN AND THE DESIGN ARE THE PROPERTY OF THE DODSTONE GROUP AND AS SUCH ARE PROTECT BY STATE AND FEDERAL COPYRIGHT LAWS.
	2. To the best of our knowledge, the Structural Drawings and these Specification Notes comply with the applicable requirements of the governing Building Code.	provide this report prior to construction.	Provide reinforcing steel placer with a set of Structural Drawings for field reference. Inspect reinforcing steel placing from structural drawings.	REPRODUCTION OR USE OF THESE DRAWINGS OTHER THAN FOR THE PROJECT INTENDED WITHOUT WRITTEN CONSENT FROM THE DODSTONE GROUP IS PROHIBITED.
	3. Construction is to comply with the requirements of the governing Building Code and all other applicable Federal, State, and Local Codes, Standards, Regulations and Laws.	 Subgrade and foundation bearing preparation shall be field controlled and tested by an experienced Licensed Engineer in accordance with the Geotechnical Report. At completion, that Engineer shall prepare and submit to the Owner, Architect, Contractor and Structural Engineer a signed and sealed letter indicating that the recommendations of the Geotechnical Report 	CONCRETE MASONRY 1. Construct masonry in accordance with ACI 530/ASCE 5, "Building Code Requirements for Concrete Masonry Structures"; and ACI	
	4. The Structural Documents are to be used in conjunction with the Architectural Documents. If a conflict exists, the more stringent governs.	have been followed.	530.1/ASCE 6, "Specifications for the Design and Construction of Load-Bearing Concrete Masonry".	PROJECT FOR:
	5. Details labeled "typical" apply to all situations that are the same or similar to those specifically referenced, whether or not they are keyed in at each location. Questions regarding the applicability of typical details shall be resolved by the Architect.	EXCAVATION, BACKFILL AND DEWATERING 1. The Contractor is solely responsible for all excavation procedures including lagging, shoring, and protection of adjacent property,	2. The structure is supported by bearing walls, u.o.n. Erect masonry prior to casting concrete columns within bearing walls or casting beams and slabs supported by bearing walls.	LEON COUNTY
	6. Openings shown on Structural Drawings are only pictorial. See the Architectural and M.E.P. drawings for the size and location of openings in the structure.	structures, streets and utilities in accordance with the requirements of the local building department and OSHA regulations. Do not excavate within one foot of the angle of repose of any soil bearing foundation unless the foundation is properly protected against settlement.	3. Use 50% solid, nominal 8x8x16, concrete masonry units conforming to ASTM C90. Block net area compressive strength shall be 1900 psi. Determine the min. F'm of 1500 p.s.i. by the unit test method or prism method. Lay up units in running bond. Sawcut units which are not in multiples of 8". Units shall be at least 8" long. Bond corners by lapping ends 8" in successive vertical	E FORT BRADEN
	7. Contractors who discover discrepancies, omissions or variations in the Contract Documents during bidding shall immediately notify the Architect. The Architect will resolve the condition and issue a written clarification.	2. The Contractor is responsible for the disposal of all accumulated water in a manner that does not inconvenience or damage the	courses. Design of walls is based on a f'm of 1500 psi.	COMMUNITY PAR
	8. The General Contractor shall coordinate all Contract Documents with field conditions and dimensions and Project Shop Drawings prior to construction. Do not scale drawings; use only printed dimensions. Report any discrepancies in writing to the Architect	SLABS ON GRADE	4. Use Type S mortar in accordance with ASTM C270 except use Type M mortar below grade. Head and bed joints shall be 3/8" for the thickness of the face shell. Webs are to be fully mortared in all courses of piers, columns and pilasters; in the starting course; and where an adjacent cell is to be grouted. Remove mortar protrusions extending 1/2" or more into cells to be grouted.	COMFORT STATIC
	prior to proceeding with work. Do not change size or location of structural members without written instructions from the Structural Engineer of Record. 9. The Contractor shall protect adjacent property, his own work and the public from harm. The Contractor is solely responsible for	1. Refer to Geotechnical Report for subgrade preparation more than 12" below bottom of slab.	5. Use standard (9 gauge) horizontal joint reinforcing in every other course. Joint reinforcing and anchors in exterior walls shall conform to ASTM A 153 Class B2, with a coating thickness of 1.50 oz/sf; conform to ASTM A 641 in interior walls. Overlap	ADDITION
	 The Contractor shall protect adjacent property, his own work and the public from harm. The Contractor is solely responsible for construction means and methods, and jobsite safety including all OSHA requirements. 	 Above subgrade, use fill containing not more than 10% passing #200 sieve and maximum 1 inch diameter. Compact to 95% of maximum dry density as determined by modified proctor ASTM D-1557. Each layer of fill shall not exceed 6" loose thickness. Compact prior to placement of the next layer. 	discontinuous ends 6". Use prefabricated corners and tees. Use truss type, except use ladder type in walls with vertical reinforcing.	15100 BLOUNTSTOWN H
	10. The Structure is designed to be structurally sound when completed. Prior to completion, the Contractor is responsible for stability and temporary bracing, including, but not limited to, masonry walls. Wherever the Contractor is unsure of these requirements, the Contractor shall retain a Florida Licensed Engineer to design and inspect the temporary bracing and stability of the structure.	3. Fill placement and compaction shall be monitored and accepted by the testing agency. Take a min. of one field density test (ASTM D-1556 or D-2922) for each 2,500 square feet of each layer. The testing agency shall randomly select test locations.	6. Use fine grout conforming to ASTM C-476, with a minimum compressive strength of 2500 psi in 28 days. Aggregate to conform to ASTM C404 for fine grout, with slump of 8" to 10". Grout all masonry containing reinforcing, all cells of 4 hour rated walls, and where indicated on the drawings. Allow mortar to cure 24 hours prior to grouting. Provide cleanout openings at the base of cells	Tallahassee, FL 32303
	11. Design Superimposed Loads: OCCUPANCY LIVE LOAD DEAD LOAD	4. For interior slabs place 10 mil polyethylene sheeting between soil and bottom of slab. Do not use any sheeting below exterior concrete slabs. Sheets shall be lapped 6" minimum and taped. Seal all penetrations, including pipes, with tape.	containing reinforcing steel to clean the cell and to tie the vertical bar to the dowel. In high-lift grouting, use 5'-0" (max.) lifts, with 1/2 hour to 1 hour between lifts. Vibrate each lift and reconsolidate the previous lift.	D SUBCONSULTANT
	Roof 20 psf 7 psf * Public Areas 100 psf 5 psf Storage 125 psf 5 psf	5. Use 4" thick slabs on grade reinforced with 6 x 6 - W2.0 x W2.0 welded wire reinforcement supplied in flat sheets only. Lap two mesh spacing's. Use chairs to support wire fabric in the center of slab.	7. Use ASTM A-615 Grade 60 reinforcing steel. Reinforce walls where indicated on the drawings and at all intersections, each side of openings and at the ends of walls. Use bar spacers at 10 ft. o.c. where grout pour height exceeds 10 ft	
	Mechanical or Electrical Room 125 psf 5 psf	6. Provide crack control joints at 10 feet maximum to limit areas between joints to 100 sq. ft. in all floating slabs on grade. Locate	8. At bond beam corners and intersections, place 1 #5 x 5'-0" T & B corner bar, with 30" legs each way, at the exterior face.9. Reinforced masonry wall construction shall be inspected by an Engineer or Architect in accordance with ACI 530.1/ASCE 6.	
	* Includes Steel Roof Deck but not Joists or Beams. Design superimposed dead loads listed above do not include masonry walls or other concentrated loads. See architectural drawings for these loads.	to conform to bay spacing whenever possible, add crack control joints at re-entrant corners which tend to invite cracks. 7. In sidewalks and walkways, locate isolation joints at 20 ft. o.c. maximum score and tool between isolation joints in equal bays of	10. Where anchor bolts, wedge anchors or anchors set in epoxy are set in a masonry wall, fill cells with grout for bolted course, one course above and two courses below.	BLISS & NYITRAY, INC. STRUCTURAL ENGINEERS Certificate of Authorization No.
	13. Design Wind Loads	5 ft. or less.	11. Provide lintels or headers with min. 8" bearing over all masonry openings.	227 N. Bronough St., Suite 7300 Tallahassee, Florida 32301 Tel. (850) 222-4454 Fax (850) 222-9429
	Governing Code ASCE 7-10 Basic Wind Speed V = 120 mph	8. See the Architectural Drawings for slab on grade depressions and other requirements. REINFORCED CONCRETE	12. Use pressure-treated wood for wood in contact with masonry.	www.bniengineers.com BNI Job No. 13T04
	Building Risk Category II Directionality Factor Kd = 0.85	 Provide Structural Concrete and concreting practices complying with ACI 301 and 318. Provide structural concrete proportioned in accordance with ACI 318, Ch. 5.3, with a minimum ultimate compressive design strength in 28 days as follows: 	CHEMICAL ADHESIVE FOR ANCHORING REINFORCING BARS, THREADED BARS AND ANCHOR BOLTS	
	Exposure- C Internal Pressure Coefficient GCpi = +/-0.18 Mean Roof Height 12 feet		 Use an epoxy, acrylic or polyester resin adhesive system such as the Hilti Hit HY150, ITW Ramset/Red Head Epcon A7 or C6 Injection System, Powers Power-Fast + System, Simpson Strong-Tie AT or ET, Allied Fastener Allied +, or accepted equivalent. Follow Manufacturer's Specifications for use and installation. 	C T OADDOZA
	14. Design Seismic Loads	Element Strength	2. Confirm the absence of reinforcing steel by drilling a 1/4" diameter pilot hole for each anchor. Do not cut reinforcing steel	DRAWN BY T. CARDOZA CHECKED BY C. CHILDERS
	SHOP DRAWINGS AND OTHER SUBMITTALS	Footings 3000 psi Slabs on Grade 3000 psi	without approval of the Structural Engineer.	DATE SUBMISSION TITLE
	 Contractor to submit shop drawings to Owner for review. Incomplete submittals will be returned without review. Submit specific components, such as columns, footings, etc., in a single package. Submit similar floors together. 	Submit statistical data, not more than one year old, for each class of concrete.	3. Refer to manufacturer's installation instructions for appropriate drill size. Thoroughly clean hole including removal of dust prior to filling with epoxy.	04-04-2013 CONSTRUCTION DOCUME
	3. On first submittal, clearly flag and cloud all differences from the Contract Documents. On resubmittals, flag and cloud all changes and additions to previous submittal; only clouded items will be reviewed.	3. Use normal weight concrete for all structural members. u.o.n. 4. Aggregates are to be clean free of chlorides, and complying with ASTM C23 gradations. Maximum coarse aggregate size 1" (C23)	 Provide anchor embedment, spacing and edge distance as shown on the Drawings. Threaded rods are A-36 galvanized steel, u.o.n. 	
	 Submittals for special structural, load-carrying items that are required by Codes or Standards to resist forces must be prepared by, or under the direct supervision of, a Delegated Engineer. Examples include Precast Concrete and Exterior Enclosure Systems. 	4. Aggregates are to be clean, free of chlorides, and complying with ASTM C33 gradations. Maximum coarse aggregate size 1" (C33 size 57). Minimum F.M. for fine aggregate is 2.4. Use admixtures conforming to ASTM C494. Provide type A, D, F or G admix in all structural concrete and Type D or G when ambient temperature is 85 degrees or higher. Design concrete, with strengths less than	WOOD CONSTRUCTION	DATE No. REVISION TITLE
В	5. A Delegated Engineer is defined as a Florida Licensed Engineer who specializes in and undertakes the design of Structural Components or Structural Systems included in a specific submittal prepared for this Project and is an employee or officer of, or consultant to, the Contractor or Fabricator responsible for the submittal. The Delegated Engineer shall sign, seal and date the submittal, including calculations and drawings. See Specifications for more specific criteria.	 4500 psi, with 4% +/- 1% entrained air, except for trowel finished floors. DO NOT USE CALCIUM CHLORIDE IN ANY CONCRETE. 5. Obtain composite samples of fresh concrete according to ASTM C172. Obtain at least one composite sample for each 100 cu. yd. or fraction thereof of each concrete mix placed each day. For slabs, increase frequency to each 50 cu. yd. or fraction placed each day. Take a slump test, ASTM C143, for each composite sample. Maximum permissible slup is 5 inches. Cast and laboratory cure one set of four compression test standard cylinders, per ASTM C31, for each composite sample. For pumped concrete, take 	1. All wood construction and connections shall conform to AITC "American Institute of Timber Construction" Manual, and the "National Design Specifications" for Wood Construction, 2005 Edition, and Florida Building Code, Chapter 23.	В
			2. Al member sizes are to be as shown on drawing and provide the following minimum properties: Member Species Fb (PSI) E (PSI)	
	6. The Trade Contractor is responsible for confirming and correlating dimensions at the job sites, for tolerances, clearances, quantities, fabrication processes and techniques of construction, coordination of the work with other trades and full compliance with the Contract Documents.	sample at point of placement. Test, per ASTM C39, one cylinder at 7 days and three at 28 days. If one of the first two 28 day tests falls below the specified strength, test the remaining cylinder at 56 days. No concrete test will be accepted if concrete is tampered with in any way after sample is obtained. Repeat sampling if water is added after initial sample is obtained.	Joists So. Pine No. 2 1500 1,400,000 Studs So. Pine No. 2 1500 1,400,000	PROFESSIONAL REGISTRATION
	7. The General Contractor/Construction Manager shall review and approve submittals and shall sign and date each drawing prior to submitting to the Architect. This approval is to confirm that the Submittal is complete, complies with the Submittal Requirements and is coordinated with field dimensions, other trades, erection sequencing and constructability.	6. Provide ASTM A-615 Grade 60 reinforcing steel. Reinforcing shall be accurately placed, rigidly supported and firmly tied in place, with appropriate bar supports and spacers. Conform to ACI 301, 315, 318 and CRSI Manual of Standard Practice. Lap continuous	Other So. Pine No. 2 1500 1,400,000 3. All wood in contact with concrete or masonry shall be pressure treated.	
	8. The Structural Engineer reviews submittals to confirm that the submittal is in general conformance with the design concept presented in the Contract Documents. Quantities and dimensions are not checked. Notations on submittals do not authorize	reinforcing 48 bar dia. Lap bottom steel over supports and top steel at midspan (u.o.n.). Hook discontinuous ends of all top bars and all bars in walls, u.o.n. provide cover over reinforcing as follows:	4. All bolts and bolted connections shall conform to ASTM A307. Use washers between wood and all bolt heads and nuts.5. All metal wood connectors shall be hot dip galvanized and shall be manufactured by Simpson Strong Tie Co., or approved equal.	
	changes to the contract sum. Checking of the submittal by the Structural Engineer shall not relieve the Contractor of responsibility for deviations from the Contract Documents and from errors or omissions in the submittal.	Element Bottom Top Sides Footings 3" 2" 3" Slabs on Grade 2" 1" 2"	6. All joists shall be laterally supported at ends by solid blocking.	Christopher S. Childers, P.E. Fla. Reg. No. 50812
	9. In addition to the above, the Structural Engineer's review of Delegated Engineer submittals is limited to verifying that the specified structural submittal has been furnished, signed and sealed by the Delegated Engineer and that the Delegated Engineer has understood the design intent and used the specified Structural Criteria. No detailed check of calculations will be made. The	Slabs on Grade 2" 1" 2"7. Where specified, provide plain, cold-drawn electrically-welded wire reinforcement conforming to ASTM A-185. Supply in flat sheets only. Lap splice one cross wire spacing plus two inches.	7. Where beams or columns are formed of two (2) or more members, they shall be full length and fastened together per Chapter 23 of the Florida Building Code.	DRAWING TITLE STRUCTURAL NOTES
	Delegated Engineer is solely responsible for his/her design, including but not limited to the accuracy of his/her calculations and compliance with the applicable codes and standards.		END OF NOTES	
	1 2	1	6	<u> </u>







FORT BRADEN PARK STORAGE

TAX ID No. 23-34-20-801-000 TALLAHASSEE, FLORIDA

PREPARED FOR:

LEON COUNTY
(DODSTONE GROUP ARCHITECTS)

INDEX OF PLANS	
2 3	KEYSHEET EXISTING CONDITIONS SITE PLAN





LEON COUNTY 2280 MICCOSUKEE ROAD TALLAHASSEE, FL. 32308

ENGINEER OF RECORD:

LOCATION OF PROJECT

TALLAHASSEE, FLORIDA

SITE PLAN INOVIA CONSULTING GROUP 1983 CENTRE POINTE BLVD., SUITE 103 TALLAHASSEE, FLORIDA 32308 MR. RUSSELL T. LARGE, P.E. No. 53933

PARCEL INFORMATION:

LOCATION: LEON COUNTY
TAX ID No. 23-34-20-80I-0000
ZONING: LAKE TALQUIN/URBAN FRINGE
CURRENT LAND USE: SPORTS COMPLEX
FRONTAGE STREET: STATE ROAD No.20
STREET MAINTENANCE: FDOT
WATER SERVICE: WELL
SEWER SERVICE: SEPTIC
ELECTRIC SERVICE: TALQUIN

PROJECT NOTES

GOVERNING STANDARDS AND SPECIFICATIONS:

FLORIDA DEPARTMENT OF TRANSPORTATION DESIGN STANDARDS DATED
FISCAL YEAR 2012/2013 AND STANDARD SPECIFICATIONS FOR ROAD AND
BRIDGE CONSTRUCTION, 2010 EDITION, AS AMENDED BY CONTRACT DOCUMENTS.

ATTENTION IS DIRECTED TO THE FACT THAT THESE PLANS MAY HAVE BEEN ALTERED IN SIZE BY REPRODUCTION. THIS MUST BE CONSIDERED WHEN OBTAINING SCALED DATA.

NO.	BY	REVISIONS	DATE
·			



1983 CENTRE POINTE BLVD., SUITE 103
TALLAHASSEE, FL 32308
PHONE (850) 298-4213 / Fax (850) 298-4498
CA8225

F	PLAN RELEASE STATUS	
	DD CODECC DDWT	
	PROGRESS PRINT	
	PERMIT RELEASE	
	BID RELEASE	
	CONSTRUCTION RELEASE	RUSSELL T. LARGE, P.E.
RFI F	FASE DATE: 30 May 13	PE NO.: 53933

PROJECT NAME: FORT BRADEN PARK STORAGE (INOVIA PROJ. NO.: 264.0

